

City Plan Services P/L 30 075 223 353

CONSTRUCTION CERTIFICATE NO. 200618/1

Issued under Part 6 of the Environmental Planning and Assessment Act 1979

APPLICANT

Name of person having benefit of the development consent: Ros Petteno

Address:

Contact Details:

DEVELOPMENT CONSENT

Consent Authority/Local Government Area:

Development Consent No & Date:

PROPOSAL

Address of Development:

Lot No:

DP No:

Building Code of Australia (BCA) Classification:

Applicable version of the BCA:

Type of Construction:

Description of development:

Ku-ring-gai Council

Phone: 0420 959 064

SSD 17424905 09/12/2022

SSD 17424905-MOD 1 21/11/2023

20 Avon Road, Pymble NSW 2073

20 Avon Road, Pymble NSW 2073

Lot 1

DP 69541

Class 9b

BCA 2022

Type N/A

The Grey House Precinct development

involving demolition of existing structures and construction of a building to

accommodate the following:

junior school classrooms (years 5 and 6);

science, technology engineering and

mathematics laboratories:

health and wellbeing facilities (consulting rooms/wards);

a dance academy;

out-of-school-hours care:

a new early learning centre for 90 children

and 20 staff;

outdoor learning spaces for existing

students; and

tree removal and associated landscaping

works.

CC1 - Demolition, earthworks, foundations &

in-ground services.

\$51.332.394.30

Schedule 1

N/A

See attached Notice

Nil

Please contact Chris Michaels for any inquiries

Chris Michaels for and on behalf of

City Plan Services Pty Ltd

BDC1974

Scope of building works covered by this Certificate:

Value of Construction Certificate (Incl GST):

Plans and Specifications approved:

Fire Safety Schedule:

Critical Stage Inspections:

Exclusions:

Conditions (as per Sections 111 & 115-117 of the Environmental Planning & Assessment (Development Certification & Fire Safety) Regulation 2021):

PROJECT BUILDING SURVEYOR

CERTIFIER

REGISTRATION NUMBER

Level 6, 120 Sussex Street, Sydney NSW 2000 P +61 2 8270 3500





That I, Chris Michaels as the certifier:

- a) certify that the work if completed in accordance with the plans and specifications identified in Schedule 1 (with such modifications verified by the certifying authority as may be shown on that documentation) will comply with the requirements of the Environmental Planning & Assessment (Development Certification & Fire Safety) Regulation 2021 as referred to in Part 6 of the Environmental Planning and Assessment Act 1979; and
- b) am satisfied that, in the case where fire safety system plans and specifications have been provided, that such plans and specifications correctly identify both the performance requirements and the deemed-to-satisfy provisions of the Building Code of Australia.

DATED THIS

08

April

2024

Chris Michaels

Director

NB: Prior to the commencement of work Section 6.6 of the Environment Planning and Assessment Act 1979 must be satisfied.





SCHEDULE 1 APPROVED PLANS AND SPECIFICATIONS

1. Endorsed Architectural Plans

| Plan Title | Prepared By | Drawing No / Ref | Revision | Date |
|-----------------------------------|-------------|------------------|----------|----------|
| GA Plan - Level 0 | BVN | AR-B10-00-01 | Α | 15/03/24 |
| Demolition & Bulk Excavation Plan | BVN | AR-U10-XX-01 | В | 15/03/24 |

2. Endorsed Structural Plans

| Plan Title | Prepared By | Drawing No / Ref | Revision | Date |
|---|-------------|------------------|----------|----------|
| Cover Sheet | TTW | S0000 | 02 | 15/03/24 |
| Notes Sheet | TTW | S0001 | 02 | 15/03/24 |
| Shoring Plan | TTW | S0501 | 02 | 15/03/24 |
| 3D - Shoring | TTW | S0502 | 02 | 15/03/24 |
| Shoring Elevations & Sections - Sheet 1 | TTW | S0511 | 03 | 18/03/24 |
| Shoring Elevations & Sections - Sheet 2 | TTW | S0512 | 03 | 18/03/24 |
| Typical Soldier Pile Details | TTW | S0521 | 03 | 18/03/24 |
| Footing Plan | TTW | S1001 | 03 | 18/03/24 |
| Footing Sections & Details - Sheet 1 | TTW | S1041 | 03 | 18/03/24 |
| Slab on Ground OSD Elevations & Details - Sheet 1 | TTW | S6010 | 02 | 15/03/24 |

3. Endorsed Civil Plans

| Plan Title | Prepared By | Drawing No / Ref | Revision | Date |
|---|-------------|------------------|----------|----------|
| Cover Sheet, General Notes & Legends, Locality Plan & Drawing | TTW | C01 | A | 19/02/24 |
| Erosion & Sediment Control Plan | TTW | C02 | A | 19/02/24 |
| Erosion & Sediment Control Details | TTW | C03 | A | 19/02/24 |
| Overall Stormwater Plan | TTW | C04 | А | 19/02/24 |
| Level 00 Siteworks Plan | TTW | C05 | A | 19/02/24 |
| Level 01 Siteworks Plan | TTW | C06 | A | 19/02/24 |



| Plan Title | Prepared By | Drawing No / Ref | Revision | Date |
|-----------------------------|-------------|------------------|----------|----------|
| Level 02 Siteworks Plan | TTW | C07 | А | 19/02/24 |
| Bulk Earthworks | TTW | C13 | Α | 19/02/24 |
| Typical Details Sheet 1 | TTW | C20 | А | 19/02/24 |
| OSD Tank Details Sheet 1 | TTW | C50 | А | 19/02/24 |

4. Endorsed Hydraulic plans

| Plan Title | Prepared By | Drawing No / Ref | Revision | Date |
|--|-------------|------------------|----------|----------|
| Legend of Symbols and General Notes | JHA | HY-000-01 | Н | 15/03/24 |
| Hydraulic Early Works Plan | JHA | HY-100-00 | Н | 15/03/24 |
| Ground Floor Drainage Layout – Sheet 2 | JHA | HY-200-01 | Н | 15/03/24 |
| Level 01 Drainage Layout – Sheet 1 | JHA | HY-201-01 | Н | 15/03/24 |
| Ground Floor Water & Gas Layout – Sheet 2 | JHA | HY-300-01 | Н | 15/03/24 |
| Level 01 Water & Gas Layout – Sheet 2 | JHA | HY-301-01 | Н | 15/03/24 |

5. Endorsed Specifications

| Title | Prepared By | Reference | Revision | Date |
|--|--------------|-----------|----------|----------|
| Architectural Specification Demolition | BVN | Z-0201 | Issue 3 | 13/06/23 |
| Architectural Specification Earthwork | BVN | Z-0222 | Issue 3 | 13/06/23 |
| Architectural Specification Quality | BVN | Z-0160 | Issue 3 | 13/06/23 |
| Architectural Specification Site Preparation | BVN | Z-0221 | Issue 3 | 13/06/23 |
| Architectural Specification Service Trenching | BVN | Z-0223 | Issue 3 | 13/06/23 |
| Architectural Specification Quality | BVN | Z-0160 | Issue 4 | 15/03/24 |
| Concrete Specification | TTW | - | Rev 2 | 12/02/24 |
| Structural Steel Specification | TTW | - | Rev 2 | 12/02/24 |
| Civil Specification | TTW | 211007 | Rev. D | 29/05/23 |
| Hydraulic Services Specification | JHA Services | 240031 | - | 15/03/24 |





| Title | Prepared By | Reference | Revision | Date |
|-------------------------------------|------------------------------|-----------|----------|----------|
| Hydraulic Services Specification | Stantec Australia Pty Ltd | 301350239 | Rev 004 | 26/05/23 |

6. Other documents relied upon

| Title | Prepared By | Reference | Date |
|---|--|-----------------------------|----------|
| CC Application Form | Ros Petteno | CFT-505652 | 25/03/24 |
| Architectural Design Statement | BVN | SSD-1742490 5 | 22/03/24 |
| | | | |
| Structural Design Certificate | TTW | 211007 | 15/03/24 |
| Hydraulic Services Design Certificate | JHA | 240031 | 15/03/24 |
| Long Service Levy Receipt | Long Service Corporation | L0000147340 | 27/03/24 |
| Geotechnical Investigation Report | JK Geotechnics | REV 2 33775SCrpt2 | 21/04/26 |
| Sydney Water Building Plan Assessment | Diego Montelvere & Sydney Water | 1824388 | 13/02/24 |
| Planning Agreement Not Required Email Confirmation | Willowtree Planning | - | 23/02/24 |
| Peer review of fire engineering brief questionnaire | Warringtonfire | 20231127- SY230335 PR1.0 | 08/04/24 |
| DA A1, A2, A3 - NSW Government Independent Planning Commission Letter | NSW Government Independent Planning Commission | SSD-17424905 | 19/05/23 |
| DA B9 - Willowtree email correspondence confirming modified DA condition change process | Willowtree Planning | - | 21/02/24 |
| DA C7, C9 - Design Certificate Civil | TTW | 211007 | 18/03/24 |
| DA C10 - Section 7.12 Receipt | Ku-ring-gai Council | D000812320 | 05/03/24 |
| DA C10 - Section 7.12 Contribution Statement | Ku-ring-gai Council | DC24-0094 | 27/02/24 |
| DA D1 - Construction Commencement Notification | PPC | - | 03/04/24 |
| DA D1 - Notice of NOC | PPC | - | 22/03/24 |
| DA D4 - Dilapidation report - Gate 3 Survey | Waratah Property Inspections | W20379.PLC.GT3 .DLP | 05/02/24 |
| DA D4 - Dilapidation Report - Pathway Survey | Waratah Property Inspections | W20379.59P.DLP | 15/01/24 |
| DA D5 | Waratah Property Inspections | W20379.59B.DLP | 15/01/24 |



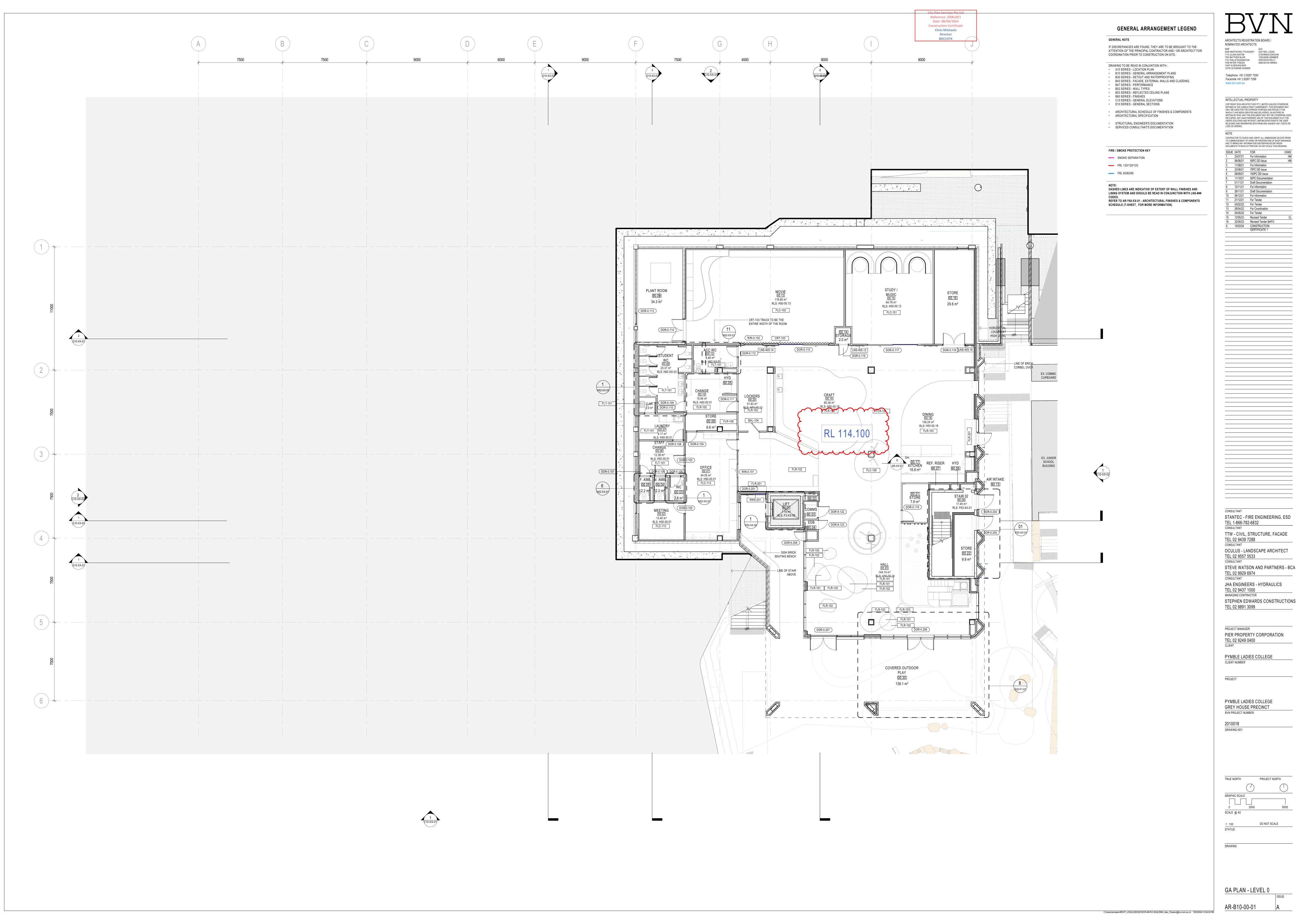
| Title | Propored By | Poforance | Data |
|---|---|-----------------------|----------|
| Title | Prepared By | Reference | Date |
| DA D5, D6, D7 - Dilapidation Survey Report - 53 Pymble Ave | Waratah Property Inspections | W20379.53.DLP | 04/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 57 Pymble Ave | Waratah Property Inspections | W20379.57.DLP | 04/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 57A Pymble Ave | Waratah Property Inspections | W20379.57A.DLP | 06/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 59A Pymble Ave | Waratah Property Inspections | W20379.59A.DLP | 04/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 61 Pymble Ave | Waratah Property Inspections | W20379.61.DLP | 04/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 77 Pymble Ave | Waratah Property Inspections | W20379.77.DLP | 04/03/24 |
| DA D5, D6, D7 - Dilapidation Survey - 79 Pymble Ave | Waratah Property Inspections | W20379.79.DLP | 04/03/24 |
| DA D5, D6, D7 - Access Attempt Letter | Waratah Property Inspections | 20379 | 12/03/24 |
| DA D5, D6, D7 - Dilapidation Report Re Qualifications Experience Letter | Waratah Property Inspections | 20379 | 12/03/24 |
| DA D5, D6, D7 - Dilapidation Survey Report | Waratah Property Inspections | W20379.53.DLP | 04/03/24 |
| DA D8 - Community Consultative Committee Meeting Notes | Community Consultative Committee | - | 14/11/23 |
| DA D9 - Community Construction Communication Strategy | WSP | Final Rev | 29/01/24 |
| DA D9 - Planning Secretary Approval of Community Communication Strategy | Department of Planning, Housing & Infrastructure | SSD-17424905- PA06 | 09/02/24 |
| DA D9 - Community Construction Communication Strategy | WSP | Rev Final | 29/01/24 |
| DA D10 - Email confirming no demolition of structures | Pier Property Corporation | - | 23/02/24 |
| DA 11 | TTW | 01 | 08/04/24 |
| - WAE stormwater plan | DDO | | 00/00/5 |
| DA D11 - Email re Stormwater Diversion Works | PPC | - | 23/02/24 |



| Title | Prepared By | Reference | Date |
|---|---|----------------------------|----------|
| DA D11 - Installation Certificate Stormwater Drainage | Paul Anderson | - | 08/02/22 |
| DA D12 - Alternative Certification Process Approval | Department of Planning & Environment | SSD-SSD- 17424905-PA-3 | 01/11/23 |
| DA D12 - Ecologically Sustainability Development | Stantec Australia Pty Ltd | Rev 3 | 27/09/23 |
| DA D15 - CEMP Approval document | Greg Hastie | - | 08/04/24 |
| DA D16 - CEMP Environmental Management Plans | Stephen Edwards Constructions | Rev B | 15/03/24 |
| DA D18 - Construction Traffic and Pedestrian Management Sub-Plan (CTPMSP) | The Traffic Planner | Revision 1.0 | 04/03/24 |
| DA D18 - Construction Traffic Management Plan | The Traffic Planner | Revision 1.0 | 06/02/24 |
| DA D18 - Email from Ku-ring-gai Council showing consultation for CTPMP | Joseph Piccoli - Ku-ring-gai Council | - | 23/02/24 |
| DA D18 - Email from Ku-ring-gai Council Traffic Engineer | Joseph Piccoli - Ku-ring-gai Council | - | 23/02/24 |
| DA D18 - Email from Ku-ring-gai Council Traffic Engineer | Joseph Piccoli - Ku-ring-gai Council | - | 13/03/24 |
| DA D19 - Construction Noise and Vibration Management Sub Plan (CNVMSP) | PWNA | 240063-PLCGH- CNVMSP-R1 | 15/03/24 |
| DA D20 - Waste Management Plan | Stephen Edwards Construction | 647 | 15/03/24 |
| DA D21 - Appendix 11 Environmental Management Plans Rev B | Stephen Edwards Construction | F106.11 20220511 Rev B | 15/03/24 |
| DA D21 - Consultation with Ku-ring-gai Council | Viola Yao - TTW | - | 06/02/24 |
| DA D21 - Consultation with Ku-ring-gai Council | Viola Yao - TTW | - | 14/02/24 |
| DA D21 - Consultation with Ku-ring-gai Council | Viola Yao - TTW | - | 19/02/24 |



| Title | Prepared By | Reference | Date |
|---|--|-----------------------|----------|
| DA D22 - Biodiversity Management Sub-Plan (BMSP) | Ecological Consultants Australia Pty Ltd | - | 26/02/24 |
| DA D24 - Emergency Response Plan | Stephen Edwards Construction | Revision A | 02/02/24 |
| DA D26 & D27 Tax Invoice Biodiversity (BCF680 | NSW Government - Biodiversity Conservation Trust | 1400000460 | 27/02/24 |
| DA D26 - Receipt of payment Section 6.33 Certificate (BCF680) | NSW Government - Biodiversity Conservation Trust | SSD 17424905 | 06/03/24 |
| DA D26 - Email acknowledgement of BCF payment | BCF Payments | - | 21/02/24 |
| DA D27 - Form 2 Charge Quote statement for BCF | NSW Government | Oct 2023 | |
| DA D28 - Department approval of Pre-Clearing Vegetation Plan | NSW Government | SSD-17424905- PA-4 | 21/12/23 |
| DA D28 Pre-clearing Vegetation Plan | Narla Environmental | Final V1.0 | 13/11/23 |
| DA D30, D31 & D32 - Statement of Compliance | Toolijooa | - | 01/02/24 |
| DA D37 - Vegetation Management Plan | Mark Bury Consulting | - | 19/01/24 |



DEMOLITION LEGEND TREES TO BE DEMOLISHED EXISTING TREES EXISTING BUILDINGS ARCHITECTS REGISTRATION BOARD / NOMINATED ARCHITECTS DEMOLITION 9356 NINOTSCHKA TITCHKOSKY 5527 NEIL LOGAN 7115 JULIAN ASHTON 2709 BRIAN DONOVAN 7053 MATTHEW BLAIR 1595 MARK GRIMMER 7151 PHILLIP ROSSINGTON 5528 DAVID KELLY 7439 PETER TITMUSS 3866 KEVIN O'BRIEN 10447 ALISON BOUNDS 10705 CATHERINE SKINNER — SITE BOUNDARY ----- NEW BUILDING LINE <u>NOTE</u> REFER TO CIVIL AND SERVICES CONSULTANTS
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REFER TO CIVIL DOCUMENTATION FOR EXTENT OF BULK EXCAVATION

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JHA ENGINEERS- HYDRAULICS 02 9437 1000 MANAGING CONTRACTOR STEPHEN EDWARDS CONSTRUCTION 02 9891 3099

PIER PROPERTY CORPORATION

PROJECT MANAGER

PYMBLE LADIES COLLEGE GREY HOUSE PRECINCT

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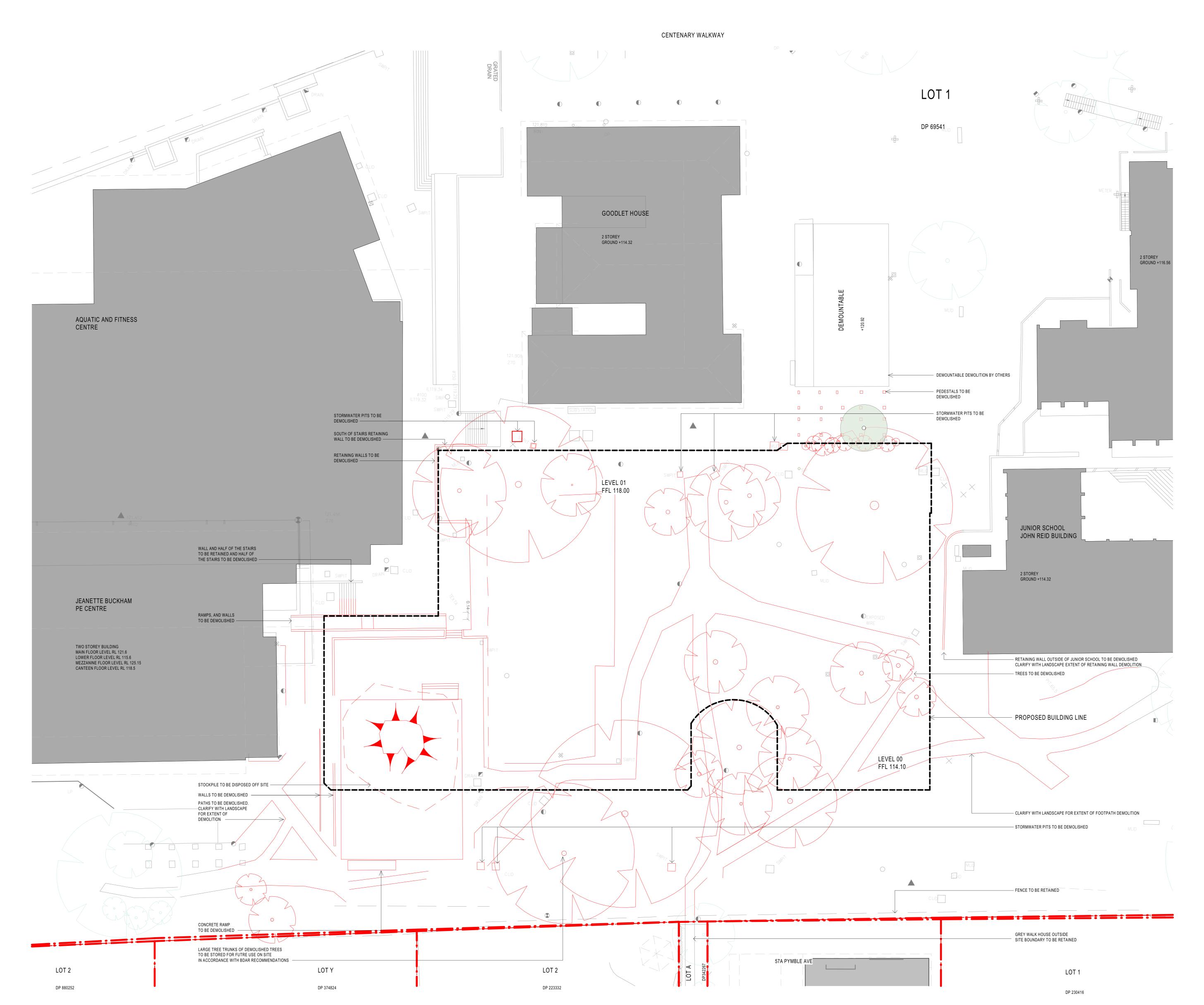
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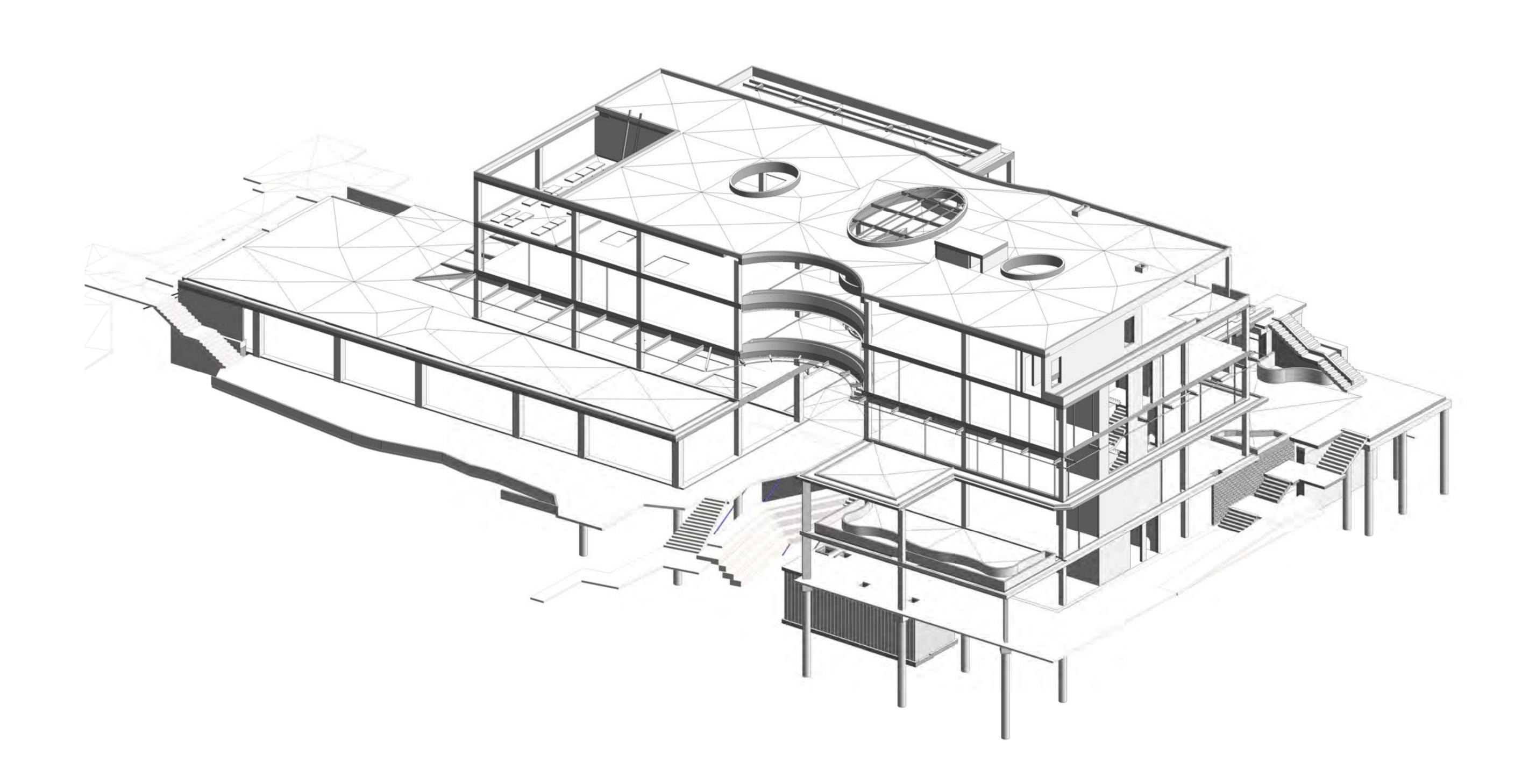
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PYMBLE LADIES COLLEGE

PYMBLE LADIES COLLEGE GREY HOUSE PRECINCT



This drawing is copyright and is the property of TTW (NSW) Pty Ltd and must not be used without authorisation. THIS DRAWING TO BE READ IN CONJUNCTION WITH ALL

| | DRAWING LIST |
|-------------|---|
| Drawing No. | Drawing Name |
| S0000 | COVER SHEET |
| S0001 | NOTES SHEET |
| S0501 | SHORING PLAN |
| S0502 | 3D - SHORING |
| S0511 | SHORING ELEVATIONS & SECTIONS - SHEET 1 |
| S0512 | SHORING ELEVATIONS & SECTIONS - SHEET 2 |
| S0521 | TYPICAL SOLDIER PILE DETAILS |
| S1001 | FOOTING PLAN |
| S1021 | RETAINING WALL SECTIONS - SHEET 1 |
| S1041 | FOOTING SECTIONS AND DETAILS - SHEET 1 |
| S2001 | COLUMN SCHEDULE |
| S2011 | COLUMN DETAILS |
| S3001 | INSITU WALL ELEVATIONS - SHEET 1 |
| S3101 | INSITU WALL DETAILS - SHEET 1 |
| S3501 | MASONRY WALL DETAILS - SHEET 1 |
| S3502 | MASONRY WALL DETAILS - SHEET 2 |
| S4001 | CONCRETE STAIR ELEVATIONS - SHEET 1 |
| S4010 | STEEL STAIR ELEVATIONS AND DETAILS - SHEET 1 |
| S4011 | STAIR DETAILS - SHEET 1 |
| S5001 | SLAB ON GROUND PLAN |
| S5021 | LEVEL 01 OUTLINE PLAN |
| S5031 | LEVEL 02 OUTLINE PLAN |
| S5041 | LEVEL 03 OUTLINE PLAN |
| S5051 | LEVEL 04 OUTLINE PLAN |
| S5061 | ROOF OUTLINE PLAN |
| S6001 | SLAB ON GROUND DETAILS - SHEET 1 |
| S6010 | SLAB ON GROUND OSD ELEVATIONS AND DETAILS - SHEET |
| S6101 | TYPICAL SUSPENDED SLAB DETAILS - SHEET 1 |
| S6201 | SLAB ON GROUND SECTIONS - SHEET 1 |
| S6211 | LEVEL 01 SECTIONS - SHEET 1 |
| S6212 | LEVEL 01 SECTIONS - SHEET 2 |
| S6221 | LEVEL 02 SECTIONS - SHEET 1 |
| S6222 | LEVEL 02 SECTIONS - SHEET 2 |
| S6231 | LEVEL 03 SECTIONS - SHEET 1 |
| S6241 | LEVEL 04 SECTIONS - SHEET 1 |
| S6251 | ROOF SECTIONS - SHEET 1 |
| S7001 | STEELWORK MEMBER SCHEDULE |
| S8101 | STEELWORK FRAMING ELEVATIONS - SHEET 1 |
| S8102 | STEELWORK FRAMING ELEVATIONS - SHEET 2 |
| S8211 | STEELWORK DETAILS - SHEET 1 |
| S9001 | LOADING DIAGRAMS - SHEET 1 |

02 ISSUED FOR CC1 01 ISSUED FOR CC1

PYMBLE LADIES COLLEGE GREY HOUSE PRECINCT

COVER SHEET

BVN 255 PITT STREET, SYDNEY, NSW 2000 AUSTRALIA

GENERAL NOTES

- 1. These drawings are for structural purposes only and are to be read in conjunction with the specification, architectural drawings, other contract documentation and the requirements of the relevant authorities. 2. Verify all setting out dimensions with the Architect.
- 3. Do not obtain dimensions by scaling the structural elements. 4. Should any ambiguity, error, omission, discrepancy, inconsistency or other fault exist or seem to exist in the contract documents, immediately notify in writing to the Superintendent.
- 5. Maintain the structure in a stable condition during construction. Temporary bracing/shoring shall be provided by the contractor to keep the structure and excavations stable at all times, ensuring that no part of the documented structure becomes overstressed. For all temporary batters
- obtain geotechnical engineer's recommendations. 6. All workmanship and materials shall be in accordance with the requirements of current Standards Australia codes and the bylaws, ordinances or other requirements of the relevant building authorities.

7. All proprietary items are to be installed and fixed in accordance with the

manufacturers specifications and instructions.

8. All work is to be carried out in accordance with all Workcover requirements and occupational health and safety act regulations 9. Construction using these drawings shall not commence until a Construction Certificate is issued by the Principal Certifying Authority.

DESIGN LOADS:

Floor loads:

Wind Loads: $V_{R} = 46$ Where R = 1000 years Region = A2 Terrain Category = 3 Earthquake Loads: Design Category = II Hazard Factor Z = 0.08 Probability Factor kp = 1.3 Importance Level = 3

Ductility $\mu = 1$

SAFETY IN DESIGN

TTW (NSW) Pty Ltd operates under Safe Work Australia's Code of Conduct for the Safe Design of Structures. These drawings shall be read in conjunction with the Taylor Thomson Whitting Transfer of Information Letter and Structural Risk and Solutions Register. Under the Code of Conduct it is the Client's responsibility to provide a copy of the Structural Risk and Solutions Register to the Principal Contractor. It is the Principal Contractor's responsibility to review the hazards and risks identified during the design process to ensure a safe workplace is maintained for the construction, maintenance and eventual demolition of the structure.

PILING NOTES

diameter shall be [600mm].

- 1. Piles are to be designed in accordance with AS2159 by the contractor for the axial loads and moments listed in the piling schedule and all requirements of the specification.
- 2. The pile design and installation shall follow the recommendations outlined in the geotechnical report No. Ref. 33775SCrpt2 prepared by JK Geotecnics. Any additional geotechnical investigation work deemed necessary shall be at the contractor's expense.
- 3. Pile spacing and pile cap design is based on [600 diameter grout injected auger piles]. Alternative pile systems may be used subject to approval. Any necessary re-design of pile caps to suit alternative systems shall be at the expense of the contractor. For single piles under columns the minimum pile
- 4. All piles or pile groups are to be centred under columns and walls UNO 5. Prior to commencing on site, the contractor must submit for approval: (a) pile type proposed (b) pile size(s), reinforcement details, founding depths and design certificate.
- The design certificate is to certify the pile design is in accordance with AS2159 for the loads listed in the piling schedule and be signed by a NER registered engineer experienced in the type of piling proposed. (c) a shop drawing setting out all pile locations from grid
- 6. The contractor is to coordinate the location of all underground services and to be responsible for ensuring that these are either avoided or relocated as
- 7. The contractor shall provide a NER registered engineer to supervise the pile
- 8. At the satisfactory completion of the work the contractor shall provide an inspection certificate signed by a NER registered engineer.

FOOTING NOTES

1. Foundations have been designed for: Allowable Bearing Pressure - Class 4 and 5 - 800 kPa Class 3 - 3500 kPa

Allowable Side Shear - Class 4 and 5 - 80 kPa Class 3 - 350 kPa

Reactivity Class M to AS 2870

2. Foundation material is to be inspected and approved by the geotechnical engineer before casting footings 3. Refer to geotechnical report No. Ref. 33775SCrpt2 dated 6 April 2021

by JK Geotechnics 4. Locate all pipes, retaining walls and excavation outside a 1:2 (vertical:horizontal) zone of influence from the bottom edge of the footing 5. Where side shear is required to be developed, clean and roughen the sides

of the excavation to the satisfaction of the geotechnical engineer. 6. Footings shall be located centrally under walls and columns unless noted 7. Footings to be constructed and backfilled as soon as possible following

excavation to avoid softening or drying out by exposure. 8. Contractor is to allow for cost of geotechnical inspections and any required certification.

RETAINING WALL NOTES

B1 0 1 2 3 4 5 6 7 8 9 10

- 1. Drainage shall be provided as shown on the drainage drawings. 2. Backfilling shall be carried out after grout or concrete has reached a minimum strength of 0.85 fc.
- Backfilling shall be approved granular material compacted in layers not exceeding 200mm to 95% Standard compaction unless noted otherwise. 3. Provide waterproofing to back of walls as specified or noted. 4. Where retaining walls rely on connecting structural elements for stability, do
- not backfill against the wall unless it is adequately propped or the elements have been constructed and have sufficient strength to withstand the loads. 5. For all temporary batters obtain geotechnical engineers recommendations.

SLAB ON GROUND NOTES

Refer to Geotechnical Report No. Ref. 33775SCrpt2 dated 6 April 2021 by JK Geotechnics for all subgrade and subbase/basecourse requirements and unless directed otherwise the following requirements apply. 1. Strip all topsoil from the construction area and remove from the site.

2. Before placing fill, proof roll exposed subgrade with 6 passes of a 10 tonne minimum roller to test subgrade and then remove soft spots (areas with more than 3mm movement under roller). Soft spots to be replaced with select fill as per table:

SELECT FILL

| Sieve Aperture (mm) to AS1152 | Percentage passed (by mass |
|-------------------------------|----------------------------|
| 75.0 | 100 |
| 9.50 | 100 to 50 |
| 2.36 | 100 to 30 |
| 0.60 | 50 to 15 |
| 0.075 | <25 |
| | |

- Plasticity index to be > or = 2% and < or = 15% - Non dispersive (a rating of nil as defined by the "dispersion" test AS1289.3.8.1) Submit proposed select fill for Engineers approval.
- 4. All basecourse material to be crushed hard rock or crushed natural gravel capable of being compacted to an even stable surface and complying with the grading and properties listed in the tables below and compacted to minimum 98% modified standard dry density in accordance with AS 1289 5.2.1

3. Compact fill areas and subgrade under buildings and pavements to minimum

Compaction under buildings to extend 2m minimum beyond building footprint.

98% standard maximum dry density in accordance with AS 1289 5.1.1.

NON-FREE DRAINING BASECOURSE

| Sieve aperture (mm) to AS1152 | Percentage passed (by mass |
|-----------------------------------|----------------------------|
| 26.5 | 100 |
| 19.0 | 95 to 100 |
| 13.2 | 75 to 90 |
| 9.50 | 60 to 90 |
| 4.75 | 42 to 76 |
| 2.36 | 28 to 60 |
| 0.425 | 10 to 28 |
| 0.075 | 2 to 10 |
| Diagnicity indays Natawastan Hann | 400/ |

- Plasticity Index: Not greater than 10% Liquid Limit: Not greater than 25% - California Bearing Ratio: Not less than 35%
- Unsound rock: Not greater than 20% Nondispersive (a rating of nil as defined by the dispersion test AS1289.3.8.1) - Submit proposed basecourse for Engineers approval.

FREE DRAINING BASECOURSE Sieve aperture (mm) to AS1152 Percentage passed (by mass)

| re aperture (IIIII) to AST 152 | reiceillage p |
|--------------------------------|---------------|
| 9.50 | 100 |
| 6.70 | 95 to 98 |
| 4.75 | 58 to 78 |
| 2.36 | 37 to 50 |
| 1.38 | 22 to 30 |
| 0.425 | 10 to 17 |
| 0.075 | 2 to 10 |

Plasticity Index: Not greater than 3% Liquid Limit: Not greater than 25% - Coefficient of permeability: Not less than 0.1mm/sec

Nondispersive (a rating of nil as defined by the 'dispersion test' AS1289.3.8.1) - Submit proposed basecourse for Engineers approval.

5. Place sand blinding to areas where Concrete Underlays are required.

CONCRETE NOTES

EXPOSURE CLASSIFICATION: External - B1 Place concrete of the following characteristic compressive strength f'c as defined in AS 1379.

| | 0.140.4 | 0 :5 1 | N |
|---------------------------------------|------------|-----------|-----------|
| Location | f'c MPa at | Specified | Nominal |
| | 28 days | Slump | Agg. size |
| Slabs on Ground | S32 | 80 | 20 |
| Suspended Slabs and Bands | S40 | 80 | 20 |
| Walls | S40~~ | 80 | 20 |
| Footings | S32 | 80 | 20 |
| Columns | S40 | 80 | 20 |
| Piles and Shoring | | BY OTHER | S { |
| Stairs | S40 | 80 | 20 { |
| > | | | } |
| | | |) |

Culling the state of the state

- 1. Use Type 'GP' cement, unless otherwise specified. 2. All concrete shall be subject to project assessment and testing to AS 1379. 3. Consolidate by mechanical vibration. Cure all concrete surfaces as directed
- in the Specification. 4. For all falls in slab, drip grooves, reglets, chamfers etc. refer to the architect's drawings and specifications. 5. Unless shown on the drawings, the location of all construction joints shall
- be submitted to engineer for review. 6. No holes or chases shall be made in the slab without the approval of the
- 7. Conduits and pipes are to be fixed to the underside of the top reinforcement 8. Slurry used to lubricate concrete pump lines is not to be used in any
- 9. All slabs cast on ground require sand blinding with a Concrete Underlay 10. $\langle 175 \rangle$ Indicates slab or band thickness

FORMWORK

- 1. The design, certification, construction and performance of the formwork, falsework and backpropping is the responsibility of the contractor.
- 2. The proposed method of installation and removal of formwork is to be submitted to the Superintendent for comment prior to work being carried

REINFORCEMENT NOTES

1. Fix reinforcement as shown on drawings. The type and grade is indicated by a symbol as shown below. On the drawings this is followed by a numeral which indicates the size in millimetres of the reinforcement.

N Hot rolled ribbed bar grade D500N R Plain round bar grade R250N grade 500L SL Square mesh RL Rectangular mesh grade 500L

2. Provide bar supports or spacers to give the following concrete cover to all

| | reinforcement unless | s otherwise noted on drawings. |
|---|----------------------------|---|
| (| Slab on Ground Footings | - 50 top, 50 bottom, 50 sides. |
| 8 | Suspended Slabs | 25 top, 25 bottom, 30 sides. |
|) | - | 40 when exposed to weather or ground. |
| { | Footing Beams | - 50 bottom, 50 sides, refer to schedule for top to ties. |
| (| Columns | - 30 to ties and spirals. |
| 8 | Walls | - 30 generally. |
| 7 | . ~ ~ ~ ~ ~ ~ | 50 when cast directly in contact with ground |

- The contact with ground the co 3. Cover to reinforcement ends to be 50 mm UNO. 4. Provide N12-450 support bars to top reinforcement as required.
- Tension Lap UNO 5. Maintain cover to all pipes, conduits, reglets, drip grooves etc. All cogs to be standard cogs unless noted otherwise. 7. Fabric end and side laps are to be placed strictly in accordance with the

manufacturers requirements to achieve a full tensile lap. Fabric shall be laid so that there is a maximum of 3 layers at any location. FABRIC LAPS

8. Laps in reinforcement shall be made only where shown on the drawings unless otherwise approved. Refer to Reinforcement Lap table below. Gap between lapped bars to be no more than 3 bar diameters as per AS3600 clause 13.2

TENSION LAPS

| | | 32 MPa CONCRETE | |
|-------------|--------------------------------|--|----------------|
| BAR SIZE | TOP BARS IN BANDS AND BEAMS | HORIZONTAL BARS IN WALLS & TOP BARS IN SLABS > 330 THICK | ALL OTHER BARS |
| N12 | 580 | 620 | 480 |
| N16 | 800 | 920 | 700 |
| N20 | 1130 | 1240 | 950 |
| N24 | 1480 | 1590 | 1230 |
| N28 | 1850 | 1940 | 1490 |
| N32 | 2250 | 2300 | 1780 |
| N36 | 2690 | 2700 | 2080 |
| N40 | 3130 | 3130 | 2420 |
| | | | |
| | | | |

| | 40 MPa CONCRETE | | |
|-------------|--------------------------------|--|----------------|
| BAR SIZE | TOP BARS IN BANDS AND BEAMS | HORIZONTAL BARS IN WALLS & TOP BARS IN SLABS > 330 THICK | ALL OTHER BARS |
| N12 | 580 | 590 | 480 |
| N16 | 770 | 870 | 670 |
| N20 | 1050 | 1150 | 890 |
| N24 | 1370 | 1440 | 1100 |
| N28 | 1700 | 1740 | 1340 |
| N32 | 2070 | 2070 | 1590 |
| N36 | 2420 | 2420 | 1870 |
| N40 | 2800 | 2800 | 2150 |
| N40 | 2800 | 2800 | 2150 |

| | 50 MPa CONCRETE | | |
|-------------|--------------------------------|--|----------------|
| BAR SIZE | TOP BARS IN BANDS AND BEAMS | HORIZONTAL BARS IN WALLS & TOP BARS IN SLABS > 330 THICK | ALL OTHER BARS |
| N12 | 580 | 580 | 480 |
| N16 | 770 | 780 | 640 |
| N20 | 950 | 1040 | 800 |
| N24 | 1230 | 1290 | 990 |
| N28 | 1530 | 1550 | 1200 |
| N32 | 1850 | 1850 | 1430 |
| N36 | 2170 | 2170 | 1670 |
| N40 | 2500 | 2500 | 1930 |

COMPRESSION LAPS

| BAR SIZE | LAP |
|----------|------|
| N16 | 640 |
| N20 | 800 |
| N24 | 960 |
| N28 | 1120 |
| N32 | 1280 |
| N36 | 1440 |
| N40 | 1600 |

ABBREVIATIONS USED ON DRAWINGS

REINFORCEMENT LEGEND Denotes the extent of area covered by bars.

2. Denotes a change in bar shape and/or length. 3. Indicates to repeat bars tagged thus (11) etc.

LAY BARS IN DIRECTION INDICATED BY ARROW 4. Bars shown staggered on plan are to be placed alternately. 5. ALT. denotes bars of different length and/or shape to be laid alternately.

Indicates 10 bars at 250 centres plus 3 bars placed one per space centrally over

- Unless Noted Otherwise EW - Each Way Not Shown On Plan EF - Each Face NSOE Not Shown On Elevation NF - Near Face FF - Far Face Bar Lengths Vary NTS Not To Scale

POST-TENSIONED CONCRETE NOTES **GENERAL**

- 1. Submit all test certificates, theoretical extensions, calculations and shop drawing to the Superintendent as required by the specification prior to
- construction 2. All reactions from post-tensioning shall be supplied to the formwork contractor for formwork design.
 - 3. Stressing contractor is to pay particular attention to concrete compaction where ducts cross columns and at all tendon anchors and ensure that pump lines are adequately chaired and restrained so as to be kept separate from tendons and reinforcement.
 - 4. Provide mesh over bands where band depth exceeds 350mm or as required by Workcover. 5. Holes cored through post-tensioned slabs must be approved by the structural engineer in writing.

TENDONS

- 1. All Strands shall be 7 wire ordinary strands with Class 2 relaxation in accordance with AS 4672.1 and AS 4672.2 unless noted otherwise. 2. Bar shall be high-tensile alloy steel bars in accordance with AS 4672.1 and AS 4672.2 with a nominal tensile strength of 1030 MPa
- unless noted otherwise. 3. Locate and fix tendons and reinforcement as shown on the contractors drawings and co-ordinate with cast in bolts, conduits and penetrations etc. Tendon profiles shall be parabolic unless noted
- 4. Ducting for slab tendons shall be galvanised steel: - 70 x 19 for 5 x 12.7dia strand tendons
- 90 x 19 for 5 x 15.2dia strand tendons 5. Seal off all ducts and securely tape joints to prevent ingress of mortar during concreting.
- 6. The performance of the post tensioning anchorages is the responsibility of the stressing contractor and they shall provide any additional bursting reinforcement needed to meet the requirements of their post tensioning system.

TENSIONING AND GROUTING

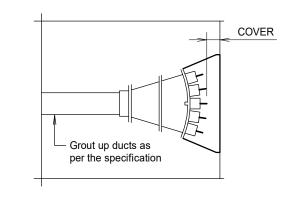
- 1. Tendons shall be stressed to jacking forces as per the contractors
- 2. The first stage of stressing is for 25% of the jacking force to be applied between 18 and 36 hours after concrete placement (f_{cp} = 9 MPa minimum) followed by the remainder of the jacking force at f_{cp} = 22 MPa unless noted otherwise below. Each individual strand or bar shall be tensioned
- during the first stage unless noted otherwise. 3. Records of net tendon elongation and other aspects of the tensioning operation required by the Specification shall be submitted to the Engineer and approved prior to cutting of tendons and grouting the ducts.

be site cured in similar conditions to the concrete element being stressed.

4. All tendons to be grouted in accordance with the specification. 5. Post-tensioning anchorage pockets shall be fully grouted with a polymer modified repair mortar. Minimum cover to any tendons or anchorage plate shall be as for the element in which they are located. 6. Concrete test cylinders used for assessing strength for tensioning are to

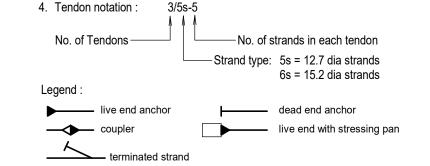
ANCHORAGE RECESS GROUTING NOT EXPOSED TO WEATHER (INTERNAL) Exposure Class A1 as per AS3600

- 1. After final stressing and approval of extensions by the engineer, cut off strands to give 30mm minimum cover to ends of strands. 2. Provide records of measured cover at each anchor recess for the engineer to inspect and provide the opportunity for the engineer to
- inspect recesses. 3. Thoroughly clean anchorage pocket (use high pressure water jet if necessary) to remove all laitance, polystyrene etc.
- 4. Prime all concrete surfaces with 'Nitobond EP' or approved 5. Grout up recess with 3:1 Sand: Cement grout mix or 'Renderoc HB'.
- Infill is to be finished flush with surrounding concrete surface. 6. The contractor shall provide records that demonstrate steps 3,4 & 5 have been satisfactorily completed at each anchor recess. ANCHORAGE RECESS GROUTING
- EXPOSED TO WEATHER (EXTERNAL) Exposure Class B1 as per AS3600 - Near Coastal/Industrial Exposure Class B2 as per AS3600 - Within 1km of coastline 1. After final stressing and approval of extensions by the engineer,
- cut off strands to give 30mm minimum cover to ends of strands. 2. Provide records of measured cover at each anchor recess for the engineer to inspect and provide the opportunity for the engineer to inspect recesses. 3. Thoroughly clean anchorage recess (use high pressure water jet if
- necessary) to remove all laitance, polystyrene etc. 4. Prime all metal surfaces with 'Nitoprime Zincrich' or approved
- 5. Prime all concrete surfaces with 'Nitobond EP' or approved 6. Grout up recess with 'Renderoc HB40' - applied as per
- manufacturers instructions. Infill is to be finished flush with surrounding concrete surface to the Superintendents requirements. A test sample is to be submitted for approval and used for acceptance/rejection criteria. 7. The contractor shall provide records that demonstrate steps 3,4,5
- & 6 have been satisfactorily completed at each anchor recess. 8. Alternative products may be used as follows: SikaTop 110 in lieu of Nitoprime Zincrich and Nitobond EP Sika MonoTop 615 in lieu of Renderoc HB40



POST TENSIONING LEGEND

1. Locate and fix tendons as shown on the drawings and co-ordinate with cast in bolts, conduits and penetrations etc. 2. Tendon profiles shall be parabolic unless noted otherwise and vertical offsets shall be measured from the deck (i.e. soffit of the concrete) to the underside of the duct, except for soffit of concrete to centreline of duct at anchorage unless noted otherwise 3. Offsets marked with a * shall be measured from the top of slab.



On the tendon lavou CENTRAL - denotes that tendon is placed flat at centre of slab NO DRAPE - denotes tendon is a straight line between ordinates

ANCHOR FIXINGS TO POST TENSIONED SLABS

1. Holes may clash with prestressing tendons at some locations so this can be accommodated by ONLY drilling bolt holes with standard MASONRY drill bits as they will not drill through steel should there be a clash (DIAMOND drill bits CANNOT be used as they will cut through tendons). Should a clash occur using a masonry drill and the tendon, then the hole should be relocated to avoid any further clash.

2. This procedure applies for drilling either from the slab soffit or top surface.

MASONRY NOTES

- 1. Temporary bracing shall be provided by the contractor to keep the masonry stable at all times.
- 2. Masonry to be in accordance with AS 3700 3. Masonry units shall comply with AS/NZS 4455 and as follows:

| Type of masonry unit | Characteristic unconfined compressive strength (f'uc) | |
|-------------------------|---|---------|
| Clay & Calcium silicate | 15 MPa | 0.8 MPa |
| Concrete | 15 MPa (hollow units) 10 MPa (solid or cored units) | 0.8 MPa |

- 4. Mortar shall consist of the following: M3 for general applications 1 part Type GP cement: 5 parts sand plus water thickener
- M4 for elements in interior environments subject to saline wetting and drying; below a damp-proof course or in contact with ground in aggressive soils; in severe marine environments; with ground in aggressive soils; in severe marine environments; in saline or contaminated water including tidal splash zones; and within 1km of an industry producing chemical pollutants. 1 part Type GP cement: 4 parts sand plus water thickener
- 5. Provide vertical control joints in masonry over permanent floor joints and as per the architectural drawings. 6. Masonry walls shown on the structural plans are load-bearing UNO. Nonloadbearing walls shall be separated from the concrete structure above with 20mm compressible filler. Masonry walls supporting slabs shall have a layer of mortar troweled smooth on top. Provide MET slipjoint to separate
- floor slabs and masonry. Provide Hercules HERCUSLIP COMPOSITE to separate roof slabs and masonry. 7. Other than what is allowed in the specification no chasing or rebates may
- be made in masonry walls without written approval. 8. The contractor shall provide records that demonstrate all masonry bed joint reinforcement, masonry ties and masonry wall stiffeners have been installed in accordance with the drawings and specification.
- 9. All load bearing concrete masonry walls shall have all cores filled with grout UNO. Core filling grout shall be thoroughly compacted. Grout to be in accordance with AS3700 and as follows: Location | fcg MPa | Specified Slump | Maximum Aggregate size Grout 20 230 10. All core filled blockwalls shall be constructed with "Double U" blocks
- of each core and shall be cleaned of mortar protrusions before grouting. 12. All core filled block walls shall have all cores filled with grout UNO. Core filling grout to be in accordance with note 9. 13. Cover to reinforcement to be 50mm to face of block UNO 14. Provide bed joint reinforcement as follows :

11. In core filled blockwalls cleanout openings shall be provided at the bottom

MET galvanized masonry reo where M3 mortar is used (supplied by DUNSTONE MAZE in NSW) Ancon stainless steel where M4 mortar is used and located as follows - in 2 bed joints below and above head and sill flashings to openings - in 2 bed joints below and above openings

SHORING WALL NOTES

- in third bed joint above bottom of wall

- in second bed joint below top of wall

GROUND ANCHORS

- 1. The design, supply, installation and tensioning of ground anchors, bolts and nails shall be carried out in compliance with the relevant Australian Standards and the Geotechnical Report. Anchorage lengths and curing times shall be determined by the
- Geotechnical Engineer. 2. Anchors, bolts and nail holes should be thoroughly cleaned and the bond grout should be allowed to cure before proof stressing.
- 3. Grouting shall conform to the requirements of AS 3600 and The Concrete Institute of Australia "Recommended Practice Z3 - Grouting of Prestressing
- 4. For proof stressing loads refer to the Geotech Report. 5. Records of all anchor extensions and test loadings are to be submitted to the Geotechnical Engineer for review.
- 6. Modifications to the arrangement shown on the drawings will require recalculation of the required working loads and shall be notified to the Geotechnical Engineer for approval. 7. Safe Working load shown is the force required after all losses of prestress,
- including draw in. 8. All anchors, bolts and nails shall be located so as to avoid all services and pits etc. The contractor is to determine the location of all services etc prior to installation of anchors.
- 9. Any variation in location or inclination of anchors, nails and bolts shall be submitted to the Geotechnical Engineer for approval. 10. For ratio of ultimate load capacity of anchor to safe working load refer to the 11. For temporary and semi-permanent anchors the length of tendon protruding
- beyond wedge grip is not to be less than 600mm to enable monitoring. 12. For corrosion protection requirements refer to the Geotechnical Report. 13. Do not destress temporary or semi-permanent anchors until the Geotechnical Engineer's approval has been obtained. 14. For temporary and semi-permanent anchors : After destressing anchors, remove anchor head and wales.

approved epoxy repair mortar. Note: this is a minimum requirement.

Contractor is to refer to Council requirements if anchors are to be fully

Cut strands at the face of pile and grout fill ducts. Make good piles with an

- PNEUMATICALLY APPLIED CONCRETE 1. Concrete to shoring walls to be pneumatically applied in one continuous
- operation. Concrete to be proportioned to achieve a batch target strength of 2. The pneumatically applied concrete shall be cured by keeping continuously wet over a period of not less than 7 days after placement or by other
- approved means. 3. Pneumatically applied concrete is to be placed by an experienced operator. 4. Pneumatically applied concrete shall conform to the requirements of the Concrete Institute of Australia Recommended Practice Z5 - Shotcreting in Australia 2020.

CONSTRUCTION SEQUENCE

8. Continue second stage as above.

- 1. Set out and drill holes for soldiers. 2. Install and plumb soldiers as detailed and backfill holes with 1: 12 - cement: sand mix.
- 3. Excavate locally and place top row of anchors as specified. 4. Place wedges on ground anchors to resist movement of wall. 5. Excavate down to horizontal CJ. 6. Place shotcrete wall as per the drawings. 7. Stress the ground anchors to Design Loads after concrete is a minimum of

STRUCTURAL STEELWORK NOTES

- 1. Provide temporary bracing to maintain stability of steelwork during construction. 2. Do not grout under base plates until first level steelwork is plumb and fixed by 3. Submit all shop drawings to the Superintendent before commencing fabrication.
- GENERAL UNO
- a. All Steelwork to be fabricated and erected in accordance with AS/NZS 5131. b. Use 10mm thick gusset, fin and end plates welded all round. c. Provide 6mm seal plate to all open ends of tube members.
- to the steel whether or not detailed on the Structural drawings. e. Studs are to be fabricated to AS1554.2 f. Shear studs (composite slab to steel) to be grade 410 MPa. Threaded studs (steel to steel) grade 380 MPa.

d. Provide all cleats and holes necessary for fixing Timber and other elements

h. Turnbuckles to be quality grade 'S' to AS2319.

- BOLTING UNO a. All bolts 20mm dia.
- b. All bolts to be grade 8.8/S.
- c. All holding down bolts to be grade 4.6. d. All bolts, including holding down bolts are to be hot dip galvanized.
- e. All connections to have a minimum of 2 bolts. f. Washers are to be in accordance with AS4100. Holes 3mm or more greater than the bolt diameter and slotted holes are
- to have plate washers of minimum 8mm thickness and are to extend a minimum of 0.5 the bolt diameter past the edge of the hole.

g. A minimum of two threads shall extend past the nut. Bolting categories are identified on the drawings in the following manner.

- 4.6/S Commercial bolts of grade 4.6 snug tightened. 8.8/S High strength bolts of grade 8.8 snug tightened. 8.8/TB High strength bolts of grade 8.8 fully tensioned to AS4100 as a bearing
- type joint with faying surfaces left uncoated. Note: Grade 8.8 bolts are NOT to be welded. /TB and /TF bolt categories shall be installed using the direct tension indicator method to AS/NZS 5131.

8.8/TF High strength bolts of grade 8.8 fully tensioned to AS4100 as a friction

WELDING UNO a. All welds to be continuous all round fillet U.N.O. as defined in table below Plate Thickness less than or equal to Weld Size 16mm 6mm 24mm 8mm

b. All welds to be category SP. c. All weld metal is to have a nominal tensile strength as defined in table below

12mm

| o. 7 iii word motal to to have a normal tenene of | a cingur do dominod in tablo bolo |
|---|--|
| Nominal yield strength of steel to be welded | Nominal tensile strength of weld metal - f _{uw} |
| All Steel with Grade ≤ 300 MPa | 430 MPa |
| All Steel > 300 MPa & ≤ 450 MPa | 490 MPa |
| Quenched & tempered steel to Grade 690 MPa | 760 MPa |

Quenched & tempered steel to Grade 690 MPa 760 MPa d. Butt weld all flanges at end plates and at all mitre cuts. e. Gussets to end plates to be butt welded.

32mm

f. All butt welds shall be full penetration, grade SP. g. Chip all welds free of slag. h. The contractor is to confirm with Architect as to where exposed welds are to be ground flush / smooth. i. Refer to Structural Steel Specification and AS/NZS 5131 for NDE weld

STEEL GRADES

Refer to Strucutral Steel Specification / Steel member schedule. PURLINS AND GIRTS

testing requirements.

- 1. All purlins to be grade AS1397/G450-G550 UNO 2. Unless noted otherwise, the fixing of purlins, girts, bridging, sheeting and any other component shall be in accordance with the manufacturer's
- specification and recommendations. 3. Sheeting / cladding is to be screw fixed to the purlins / girts to provide lateral restraint to the purlins/girts in accordance with the manufacturer's requirements. 4. All purlin and girt bolts to be in accordance with purlin manufacturers requirements

| and as defined in table below UNO. | |
|------------------------------------|-----------|
| Purlin Depth | Bolt Size |
| 100mm to 250mm | M12 4.6/S |

300mm to 350mm

Grade 8.8/S bolts to be used where nominated on drawings. 5. Provide double purlins at expansion joints in roof sheeting. 6. Purlin / girt sizes shown are based on the current BLUESCOPE LYSAGHT purlins and girts design data, including restraint from roof sheeting and bridging. The manufacturer should confirm any alternative systems used are equivalent or

M16 4.6/S

- redesign the purlins / girts to provide an equivalent system. 7. Purlin / girt cleats are to be 8mm minimum thick & in accordance with the Manufacturers details. Where the distance between the bottom flange of the purlin and the rafter is
- greater than 100mm use 75 x 75 x 8 EA cleats 8. Provide 75 x 75 x 4 duragal galvanized angle trimmers to support roof sheeting edges at all hips, valleys and angled sheet edges. Fix to each purlin with one No. 14 Tek screw. 9. Bridging shall be designed and erected in accordance with the manufacturer's

requirements. Rod bridging is not acceptable unless approved in writing.

10. For bridging members to purlins at curved roof areas provide bridging suitable for

STEELWORK FINISHES

curved roofs to manufacturer's details.

Refer to Structural Steel Specification

POST-INSTALLED ANCHORS / DOWELS / REINFORCING BAR NOTES

1. All fasteners must be installed in accordance with the manufacturer's installation instructions that may be supplemented by information specified by the design engineer. This includes setting tools and cleaning accessories prescribed by the manufacturer's installation instruction(s). 2. Installation should be performed by an AEFAC certified installer or by a person trained by the manufacturer/supplier of the specified product. 3. Refer to concrete specification for details and procedures for anchor

- substitution approval. 4. All post-installed fasteners are to be prequalified for use to AS 5216 via a European Testing Assessment ETA report or an equivalent independent assessment.
- 5. Post-installed anchors, dowels or bars shall be pull-out tested in accordance with the project specifications and/or TTW's specifications, whichever is more onerous 6. All aborted holes are to be filled with mortar equal to the base material concrete strength but not less than 40 MPa as per AS 5216.

CHEMICAL ANCHORING:

Unless noted otherwise on drawings:

All chemical / epoxy anchors to be Hilti HAS-U 8.8 galvanised rods fixed with Hilti HIT-HY 200-R V3 injection mortar. Minimum embedment depth to be as below UNO.

M12 = 110mmM16 = 125mmM20 = 170 mm

REINFORCEMENT ANCHORING:

Where drill and fix or drill and grout is called up on the drawings use Hilti HIT-RE 500 V3 or Ramset Reo502 PLUS epoxy adhesive UNO. Holes are to be cleaned and dry in accordance with manufacturer's recommendation. Embedment length is to be as specified on the detail. MECHANICAL ANCHORING:

Refer to the product type and embedment depth nominated on the detail. The removal and resetting of post installed mechanical anchors is not

8. The drilling of installation holes shall be carried out with a masonry bit that will not cut or damage reinforcement and post-tensioning. Carbide and Multi-cutter drill bits are not to be used. 9. Prior to installation undertake scanning to determine the location of any post-tensioning, reinforcement and embedded services. If installation is not possible without damaging existing embedded item, contact TTW for further

www.www.www.

HN EMC 15.03.24

HN EMC 19.02.24

Rev Description Eng Draft Date PYMBLE LADIES COLLEGE **GREY HOUSE PRECINCT**

NOTES SHEET

02 ISSUED FOR CC1

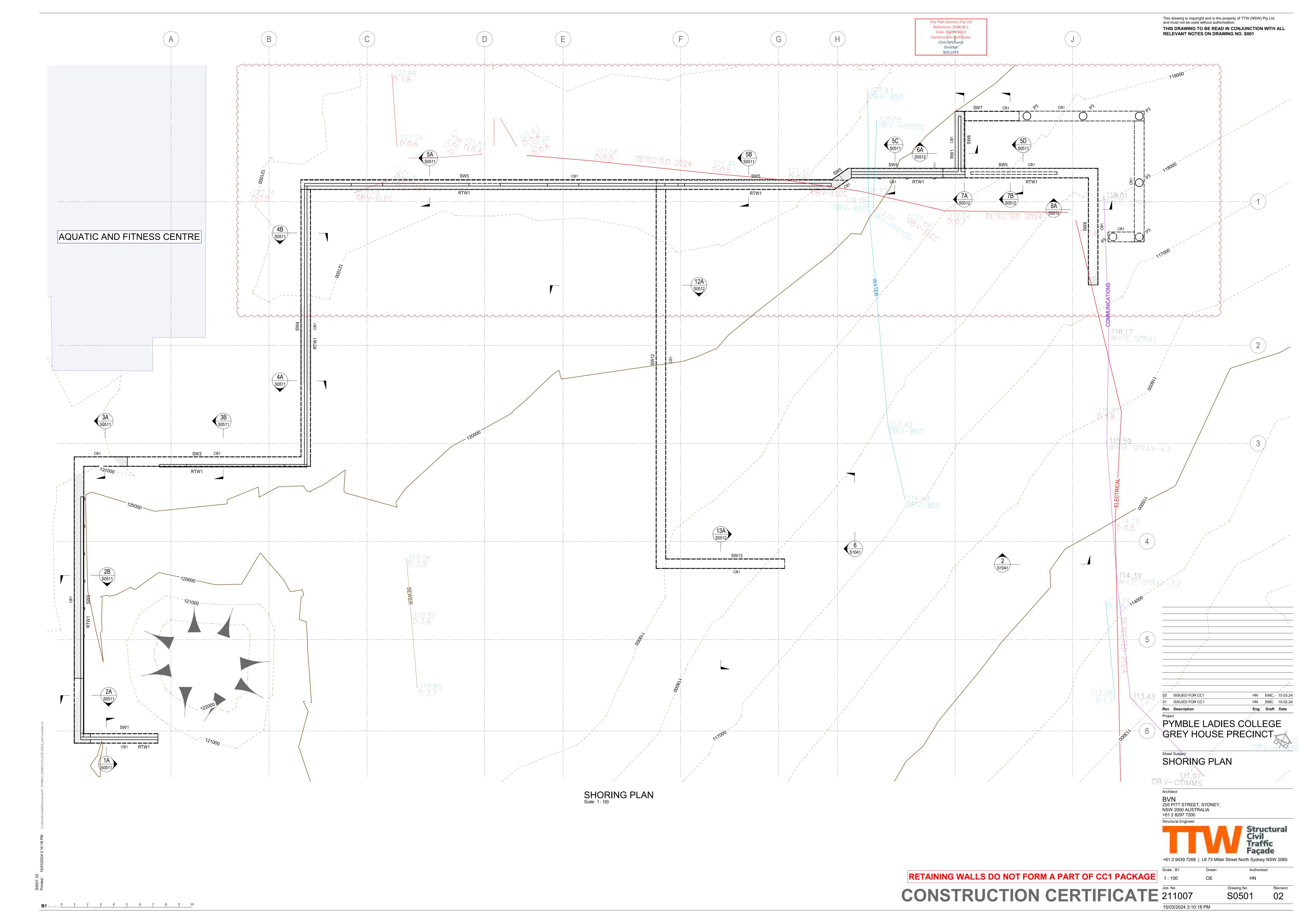
01 ISSUED FOR CC1

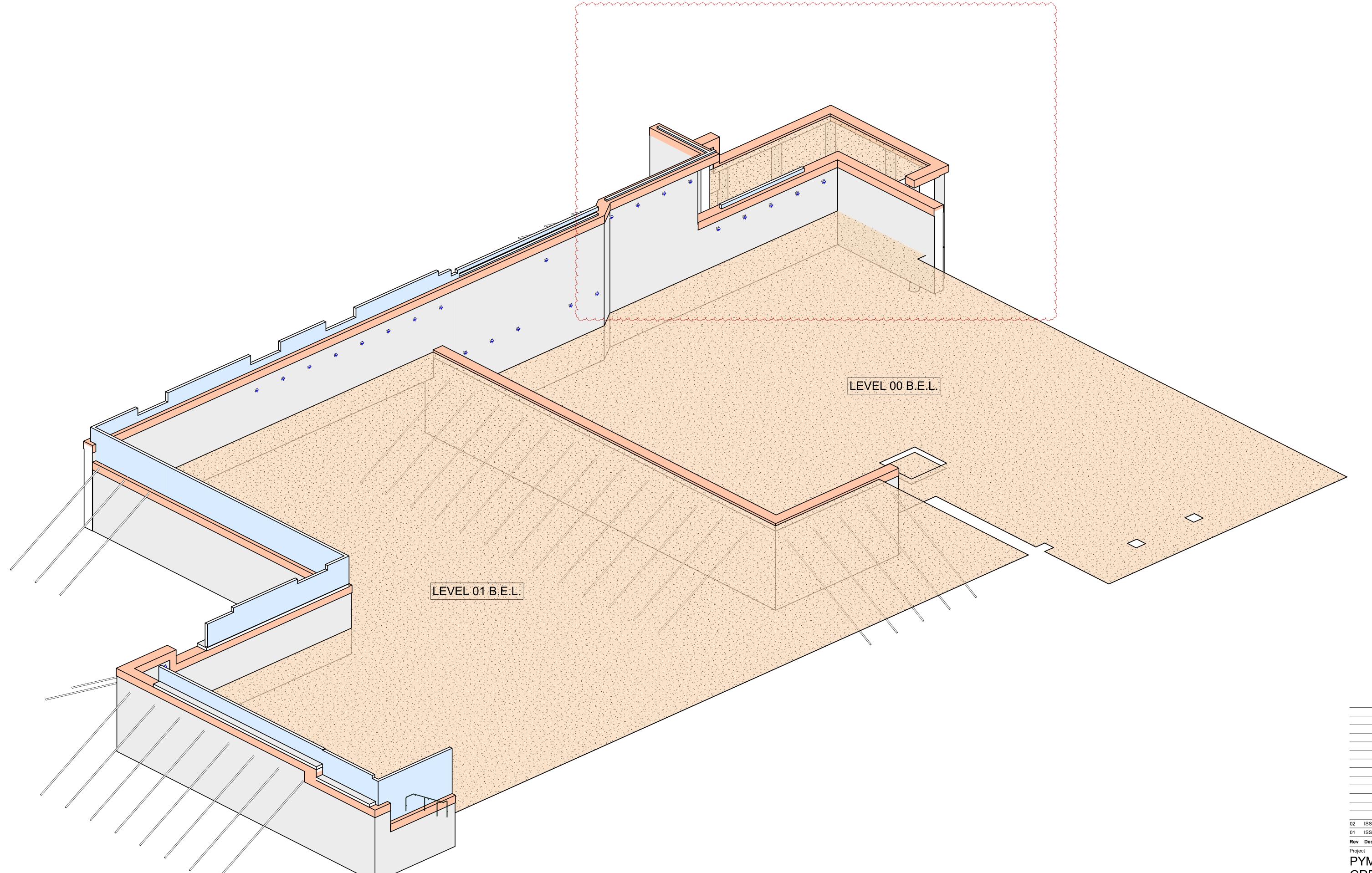
255 PITT STREET, SYDNEY, NSW 2000 AUSTRALIA +61 2 8297 7200 Structural Engineer Structural Civil

RETAINING WALLS DO NOT FORM A PART OF CC1 PACKAGE

Drawing No S0001

CONSTRUCTION CERTIFICATE 211007





02 ISSUED FOR CC1 HN EMC 15.03.24 01 ISSUED FOR CC1 Eng Draft Date

PYMBLE LADIES COLLEGE GREY HOUSE PRECINCT

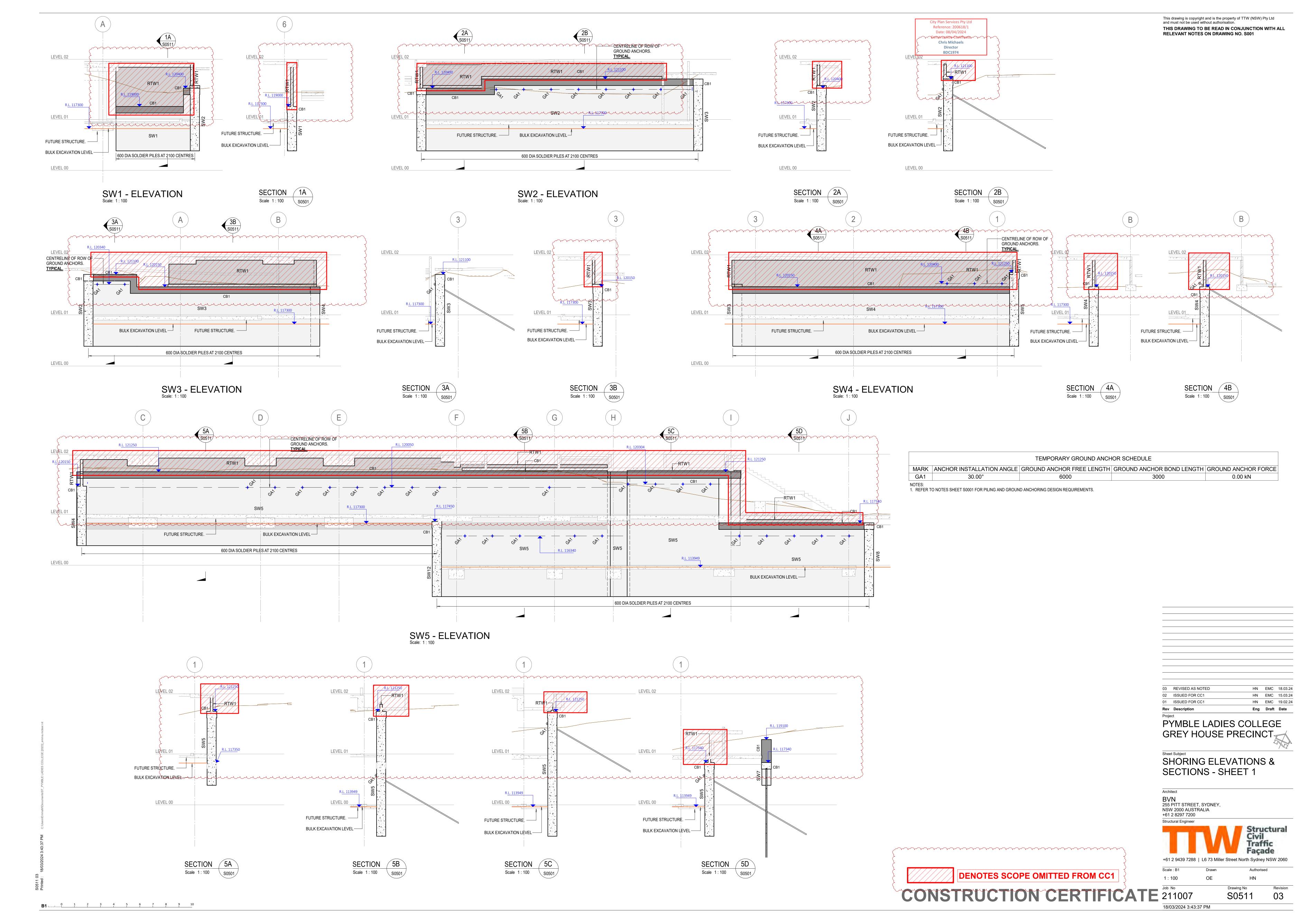
Sheet Subject
3D - SHORING

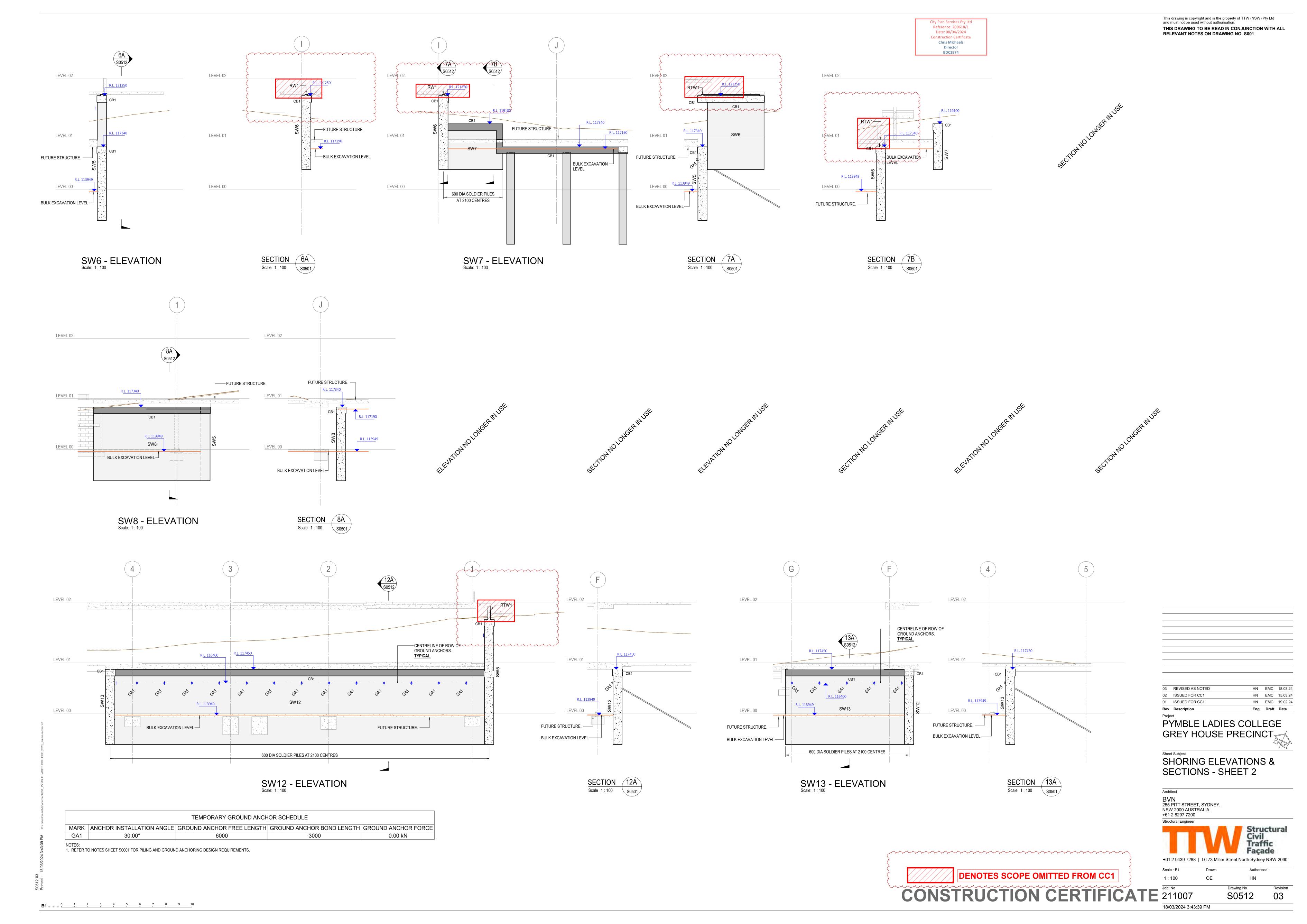
BVN 255 PITT STREET, SYDNEY, NSW 2000 AUSTRALIA +61 2 8297 7200 Structural Engineer Structural Civil Traffic Façade

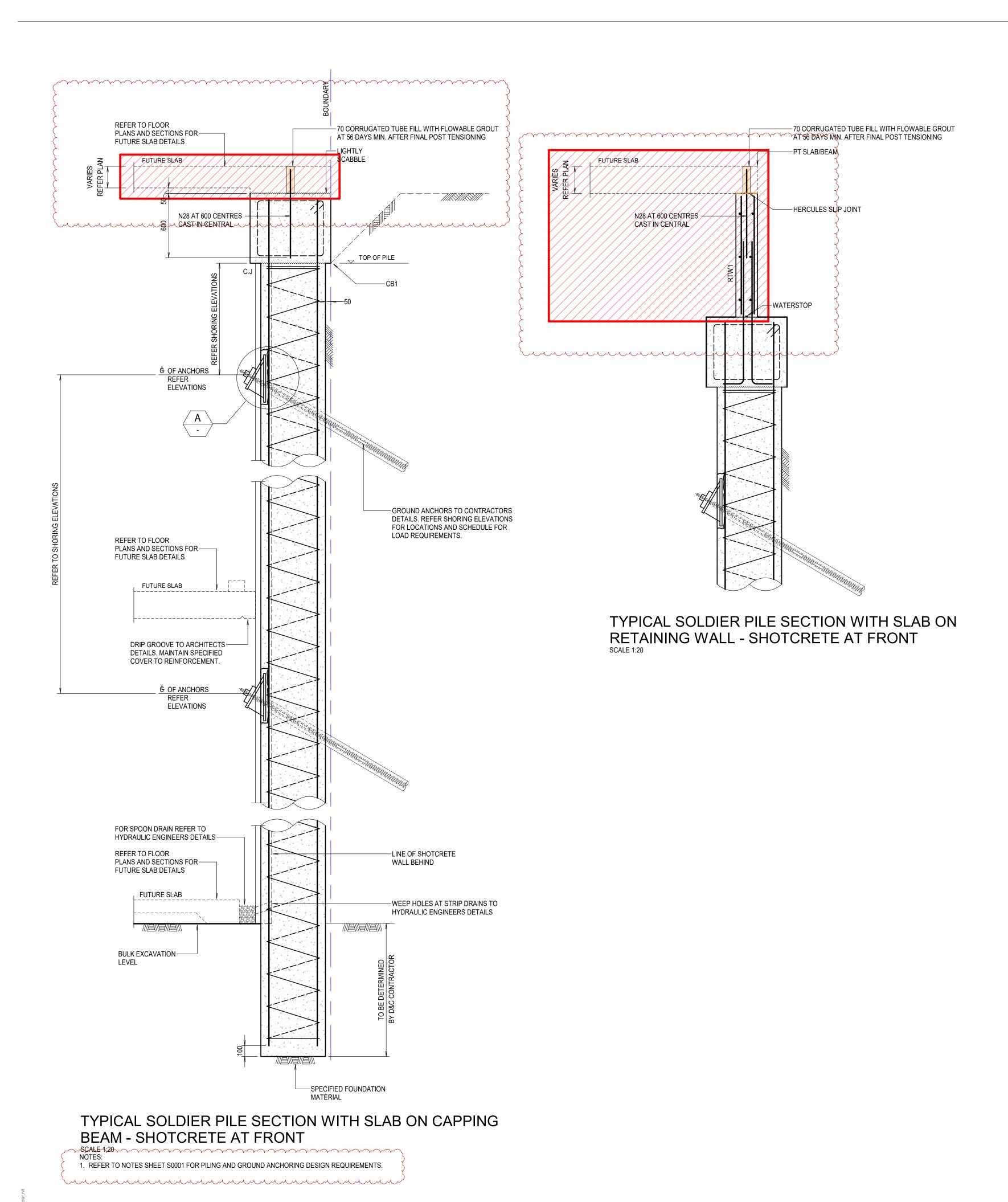
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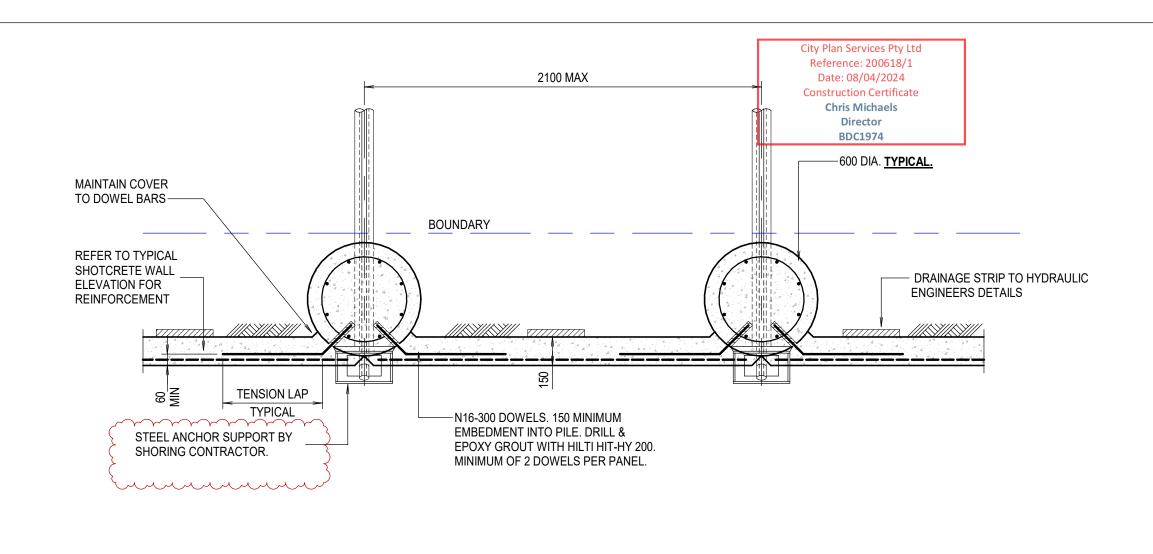
CONSTRUCTION CERTIFICATE 211007

3D - SHORING

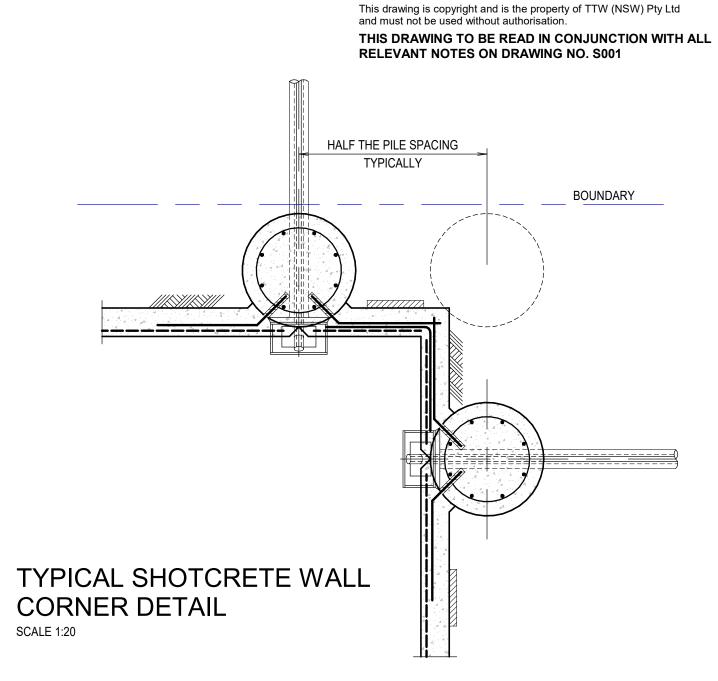








TYPICAL SHOTCRETE WALL PLAN



03 REVISED AS NOTED 02 ISSUED FOR CC1 HN EMC 15.03.24 01 ISSUED FOR CC1 HN EMC 19.02.24 Eng Draft Date

PYMBLE LADIES COLLEGE GREY HOUSE PRECINCT

TYPICAL SOLDIER PILE **DETAILS**

NSW 2000 AUSTRALIA Structural Engineer

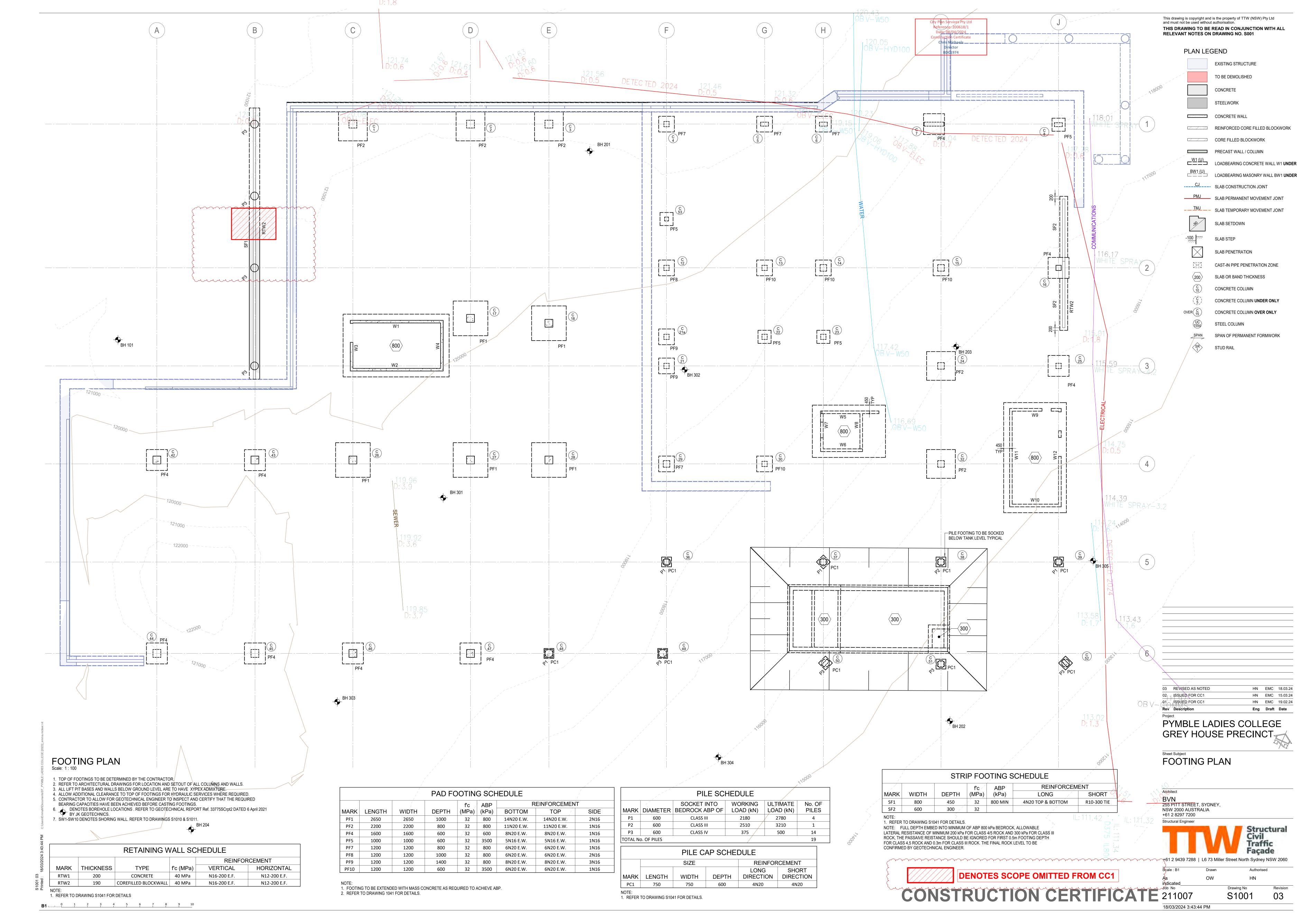
Structural Civil Traffic Façade

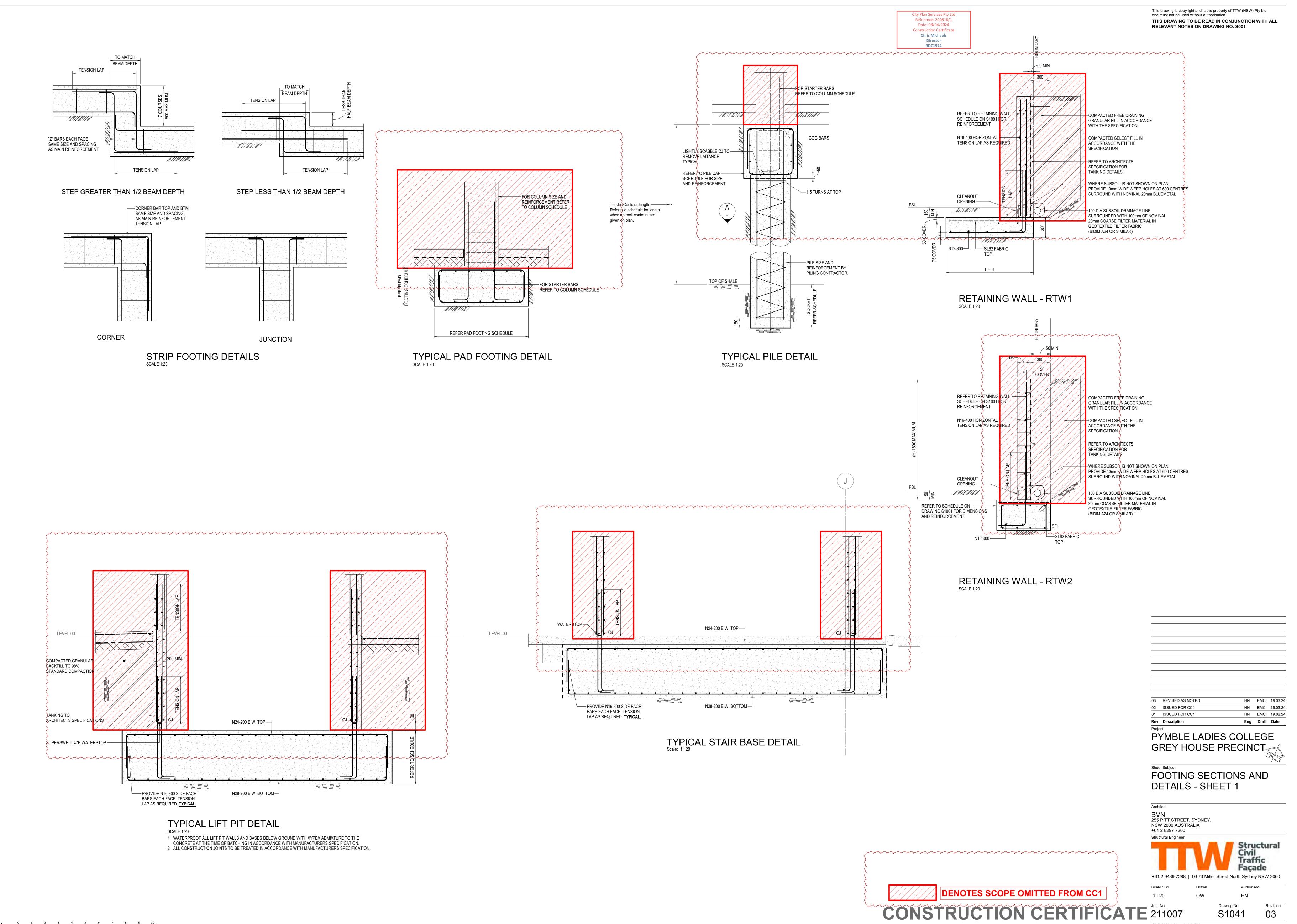
CONSTRUCTION CERTIFICATE 211007

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DENOTES SCOPE OMITTED FROM CC1

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